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FLOOD INSURANCE STUDY

ORIGINAL
COMPLETED
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VILLAGE OF ROMEOVILLE, ILLINOIS

WILL COUNTY

MAY 3, 1982

Federal Emergency Management Agency

COMMUNITY NUMBER - 170711

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Flood Profiles

Des Plaines River — Panel 01P

Flood Boundary and Floodway Map

PUBLISHED SEPARATELY:

Flood Insurance Rate Map

FLOOD INSURANCE STUDY

VILLAGE OF ROMEOVILLE,
WILL COUNTY, ILLINOIS

1.0 INTRODUCTION

1.1 Purpose of Study

This Flood Insurance Study investigates the existence and severity of flood hazards in the Village of Romeoville, Will County, Illinois, and aids in the administration of the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973. This study will be used to convert the Village of Romeoville to the regular program of flood insurance by the Federal Emergency Management Agency (FEMA). Local and regional planners will use this study in their efforts to promote sound flood plain management.

In some states or communities, flood plain management criteria or regulations may exist that are more restrictive or comprehensive than those on which these Federally-supported studies are based. These criteria take precedence over the minimum Federal criteria for purposes of regulating development in the flood plain, as set forth in the Code of Federal Regulations at 44 CFR, 60.3 (d). In such cases, however, it shall be understood that the state (or other jurisdictional agency) shall be able to explain these requirements and criteria.

1.2 Authority and Acknowledgements

The source of authority for this Flood Insurance Study is the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973.

The hydrologic and hydraulic analyses for this study were performed by Alfred Benesch and Company for the Federal Emergency Management Agency, under Contract No. H-4714. This study was completed in May 1981.

1.3 Coordination

The designation of areas requiring detailed study was completed in a meeting on May 16, 1978, attended by representatives of the Village of Romeoville, the Study Contractor, FEMA, the Illinois Division of Water Resources (IDWR) and the Will County Regional Planning Commission. On October 11, 13 and 18, 1978, a legal notice was published in the Herald News announcing the intent to perform a flood elevation study for the Village of Romeoville.

On December 9, 1981, the results of the work by the Study Contractor were reviewed and accepted at a final coordination meeting attended by representatives of the Study Contractor, FEMA, and the community.

2.0 AREA STUDIED

2.1 Scope of Study

This Flood Insurance Study covers the Village of Romeoville, Will County, Illinois. The area of study is shown on the Vicinity Map (Figure 1).

The areas studied by detailed methods were selected with priority given to all known flood hazard areas and areas of projected development or proposed construction for the next five years, through May 1986.

Approximate methods of analysis were used to study those areas having a low development potential or minimal flood hazards as identified at the initiation of the study. The scope and methods of study were proposed to and agreed upon by FEMA and the Village of Romeoville.

Flooding caused by the overflow of the Des Plaines River was studied in detail within the corporate limits. Due to the overtopping of downstream levees along the Illinois and Michigan Canal, the canal was studied by approximate methods. Approximate methods were also used to study the Chicago Sanitary and Ship Canal and ponding areas adjacent to the canals within the corporate limits.

2.2 Community Description

The Village of Romeoville is located in northernmost Will County, Illinois, approximately 30 miles from downtown Chicago. The village is bordered by the City of Lockport on the southeast and unincorporated Will County on all other sides. The community area is 2.93 square miles. The population in 1970 was 12,674, and the population estimate in 1975 was 16,051, which represents an increase of 27 per cent (Reference 1).

Romeoville is primarily a residential community with some industry along the Illinois and Michigan Canal.

The climate of the area is best described as humid continental with warm to hot summers and moderately cold winters (Reference 2). The seasons are markedly distinct and, in general, lag three to five weeks behind the solstices. The proximity of Lake Michigan to the study area has a moderating effect on the climate. The average annual temperature is about 51 degrees Fahrenheit (F.). Average monthly temperatures vary from 24

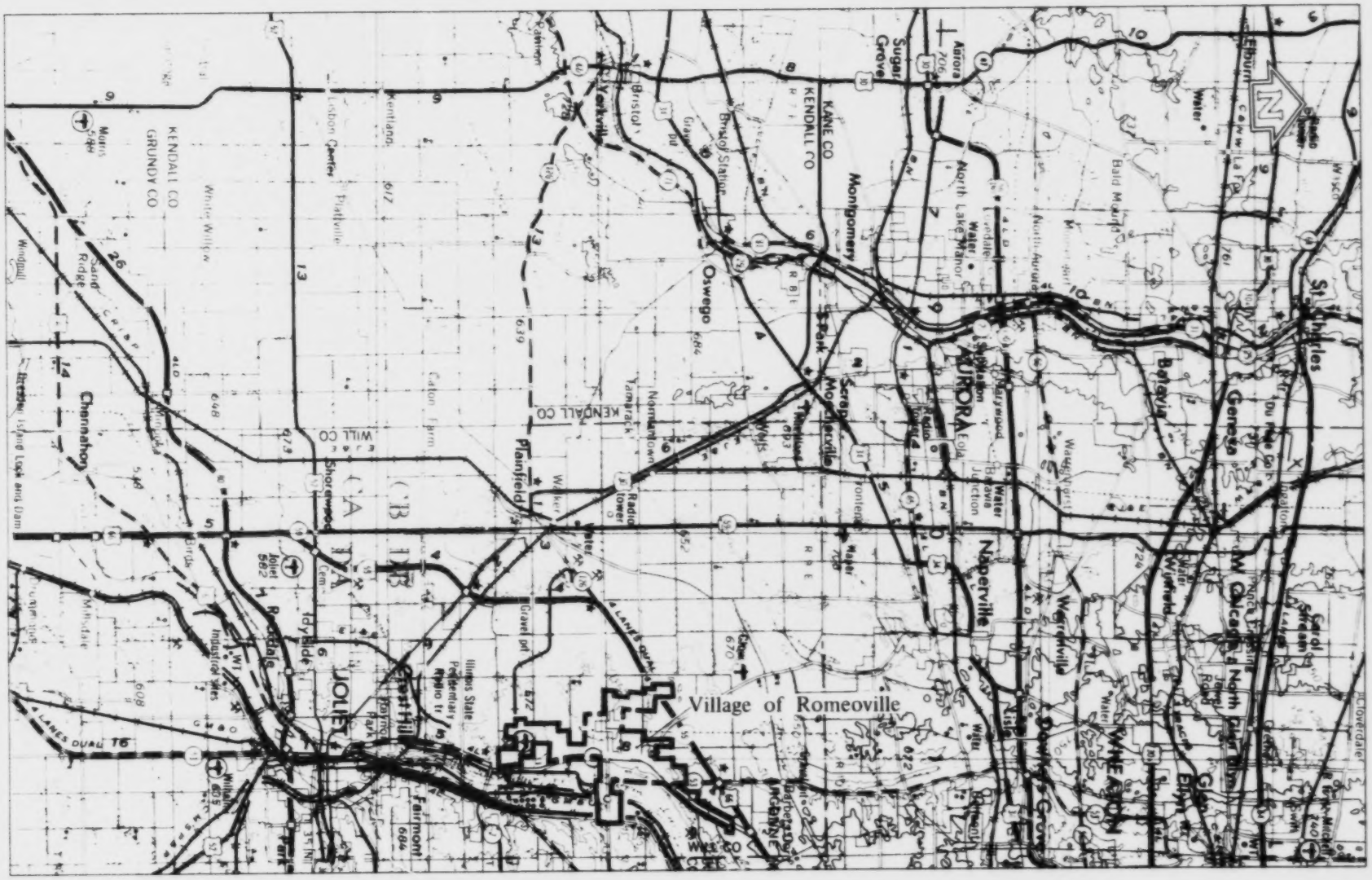

FIGURE 1

FEDERAL EMERGENCY MANAGEMENT AGENCY

VILLAGE OF ROMEOVILLE, IL

(WILL CO.)

APPROXIMATE SCALE

4 0 4 8 12 MILES

VICINITY MAP

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degrees F. in January to 74 degrees F. in July, with daily extremes ranging from -25 degrees F. to 110 degrees F. The average temperature by month is:

January	23.9	May	60.6	September	65.4
February	27.3	June	70.6	October	55.0
March	37.2	July	74.2	November	40.4
April	50.5	August	72.8	December	28.0

Precipitation within the area occurs as rain, sleet, snow and hail, with average annual precipitation amounting to 34 inches. About 60 per cent of the annual rainfall occurs from April to September. The average precipitation by month is:

January	1.64	May	3.69	September	3.46
February	1.53	June	3.83	October	2.69
March	2.62	July	3.92	November	2.09
April	3.68	August	3.14	December	1.95

The Village of Romeoville is contained within the Des Plaines River watershed. The topography of this watershed is extremely heterogeneous due to the comparatively recent Wisconsin glaciation. Romeoville can be classified topographically into two categories: ground moraine, outwash, bedrock; and moraine, front hachured. The ground moraine, outwash, bedrock is located along the Des Plaines River bed, changing to moraine, front hachured approximately two miles to either side of the river. The glacial deposits of the watershed are underlined by Silurian dolomite of Niagaran and Alexandrian Ages (References 3, 4 and 5).

Along the Des Plaines River, the soil is of the Sawmill-Romeo-Rockton Association. The characteristics of the soil are: nearly level to moderately sloping, poorly drained to well drained soils that are formed in alluvial sediments and soils formed in layers of alluvium and the underlying limestone bedrock. To the west of the river in the main residential section of Romeoville, the soil is of the Saybrook-Drummer Association. The characteristics of the soil are: deep moderately permeable, well drained and moderately well drained soils and some poorly drained soils that are formed in a thin layer of loess and the underlying loam glacial till (Reference 5).

The Des Plaines River, the Chicago Sanitary and Ship Canal and the Illinois and Michigan Canal all pass through the middle of the Village of Romeoville.

The Des Plaines River is the sixth largest tributary of the Illinois River. It enters the Will County boundary northeast of Romeoville and flows in a southwesterly direction to form its confluence with the Kankakee River in Grundy County. The total length of the Des Plaines River studied as a detailed stream in incorporated Romeoville is 1.5 miles (Reference 3).

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The Illinois and Michigan Canal, connecting Lake Michigan at Chicago with the Illinois River at LaSalle-Peru, was constructed by the State of Illinois and was opened for barge traffic in 1848. The route of the main canal extends from the South Branch Chicago River at Ashland Avenue, Chicago, southwestward to the east bank of the Des Plaines River; and then southward, generally parallel and adjacent to that stream, to the City of Joliet. At Joliet, the Illinois and Michigan Canal crosses the Des Plaines River, then parallels the Des Plaines and Illinois Rivers to its confluence with the Illinois River in LaSalle-Peru. Presently, the Illinois and Michigan Canal is not used for barge traffic. The total length of the Illinois and Michigan Canal is 96.4 miles (Reference 3). The length of the canal contained within the study reach is 3,980 feet. The dimensions of an average canal cross section are a width of 100 feet and a depth of 12 feet.

The Des Plaines River flood plain is principally undeveloped.

2.3 Principal Flood Problems

The principle causes of flooding in Romeoville are due to the overbank flooding of the Des Plaines River and surface water accumulation in depressions and lowland areas.

The overbank flooding by the Des Plaines River is generally associated with spring snowmelt combined with ice jams and frontal rainstorms. This flooding has occurred frequently, with floods having been recorded at the Riverside gage No. 05532500 upstream of Romeoville since 1919. The estimated frequency (Reference 6) of peak discharges for these historic events are presented below:

Year	Discharge cubic feet per second (cfs)	Recurrence Interval (Years)
1919	7,450	53
1948	6,510	17
1916	6,480	16
1950	6,340	14
1954	6,340	14
1920	6,110	11
1957	5,950	10
1918	5,790	9
1938	5,540	8
1960	5,500	7
1972	5,460	6

Depressed overbank land areas next to the western banks of the Des Plaines River have caused some swamping. The lowlands just west of the Illinois and Michigan Canal cause much flooding in that area. This type of flooding is largely due to inadequate drainage back to the streams.

Frequency and depth of flooding in these areas are unrelated to the water-surface elevation along the streams. Some areas are flooded only briefly after periods of heavy rainfall or snowmelt, whereas others remain inundated continuously, depending, to some extent, upon the rates of evaporation and seepage into the ground (Reference 7).

2.4 Flood Protection Measures

Presently, no protective structures exist within the Village of Romeoville and none are planned for the future.

3.0 ENGINEERING METHODS

For the flooding sources studied in detail in the community, standard hydrologic and hydraulic study methods were used to determine the flood hazard data required for this study. Flood events of a magnitude which are expected to be equalled or exceeded once on the average during any 10-, 50-, 100-, or 500-year period (recurrence interval) have been selected as having special significance for flood plain management and for flood insurance premium rates. These events, commonly termed the 10-, 50-, 100-, and 500-year floods, have a 10, 2, 1, and 0.2 per cent chance, respectively, of being equalled or exceeded during any year. Although the recurrence interval represents the long term average period between floods of a specific magnitude, rare floods could occur at short intervals or even within the same year. The risk of experiencing a rare flood increases when periods greater than one year are considered. For example, the risk of having a flood which equals or exceeds the 100-year flood (one per cent chance of annual occurrence) in any 50-year period is about 40 per cent (four in ten), and for any 90-year period, the risk increases to about 60 per cent (six in ten). The analyses reported here reflect flooding potentials based on conditions existing in the community at the time of completion of this study. Maps and flood elevations will be amended periodically to reflect future changes.

3.1 Hydrologic Analyses

Hydrologic analyses were carried out to establish the peak discharge-frequency relationships for floods of the selected recurrence intervals for each flooding source studied in detail affecting the community.

The Des Plaines River was studied in detail as part of the Unincorporated Areas of Will County and the City of Joliet Flood Insurance Studies (References 3 and 8).

Peak discharge-frequency data was obtained for the Des Plaines River by use of the U.S. Army Corps of Engineers (COE) HEC-1 computer program (Reference 9). The HEC-1 Flood Hydrograph Package was used to obtain routing coefficients and to synthesize unit hydrographs. Routing, local inflow and the combining of hydrographs for the Des Plaines River were performed using this method.

The Des Plaines and Riverside recording gages have long-term records without expensive artificial controls. Other gages used in the analysis include the Lemont, Romeo, South Holland and Lockport gages. Input hydrographs were synthesized from the storm records and the unit hydrographs. Storm records were averaged from the Mayfair, Skokie, McHenry and North Branch pumping station gages using historical rainstorm amounts.

Peak discharges were checked with those discharges obtained from frequency analyses of the Joliet gage and compared favorably. Recurrence intervals of floods, other than the 100-year flood, were computed by obtaining discharge-drainage area relationships at the gages and interpolating between gages to obtain discharge-frequency relationships for different locations along the systems.

Estimates of discharge for the reach of the Illinois and Michigan Canal upstream from Joliet were made by Harza. In this reach, discharges are limited by the bank-full capacity of the Illinois and Michigan Canal. Water which overflows the banks flows into the Deep Run basin by means of a diversion weir at the Texaco Plant and a pipe-overflow under Canal Street. Hydraulic computations were made to determine the bank-full capacity of the Illinois and Michigan Canal. Regional equations and the State Standard Method (Reference 6) were used, along with rainfall flooding depth tables for Illinois (Reference 10) to determine flood discharges and volumes available for interbasin spillage. Flood frequency relationships for the Illinois and Michigan Canal upstream from Joliet were computed using mass-balance techniques based on the channel capacity and the flood plain area available for spillage. Decreasing discharges on the Illinois and Michigan Canal are due to hydrologic losses occurring at the Texaco Diversion, Texaco Dam, 13th Street Diversion and downstream of the confluence of Fraction Run.

Peak discharges for the 10-, 50-, 100-, and 500-year floods of each flooding source studied in detail in the community are shown in Table 1.

TABLE 1 - SUMMARY OF DISCHARGES

FLOODING SOURCE AND LOCATION	DRAINAGE AREA (SQ MILES)	PEAK DISCHARGE (CFS) 10-YEAR	50-YEAR	100-YEAR	500-YEAR
DES PLAINES RIVER					
At Romeo Road	696	6,100	7,900	9,200	10,200

3.2 Hydraulic Analyses

Analyses of the hydraulic characteristics of the streams in the community were carried out to provide estimates of the elevations of the floods of the selected recurrence intervals along each flooding source studied in detail.

Cross sections for the backwater analyses were determined from field surveys with some overbank portion cross sections being determined from topographic maps (Reference 11). The locations of the surveyed cross sections were determined by examining maps of historic flooding (Reference 7) in order to find areas of changing flood width. Therefore, the cross sections are representative of the actual flow conditions. Cross section locations were at close intervals above and below bridges, dams and culverts in order to compute the significant backwater effects of these structures. All bridges were surveyed to determine structural geometry.

Locations of selected cross sections used in the hydraulic analyses are shown on the Flood Profiles. For stream segments for which a floodway was computed (Section 4.2), selected cross section locations are also shown on the Flood Boundary and Floodway Map.

Roughness coefficients (Manning's "n"), for these computations, were assigned on the basis of field inspection, analyses of photographs, engineering judgment and previous analyses by other agencies. The roughness coefficients for the Des Plaines River range from 0.035-0.038 for the channel and 0.05-0.075 for the overbank.

Flood elevations for the Des Plaines River were taken from the Unincorporated Areas of Will County, Illinois Flood Insurance Study (Reference 3). The Des Plaines River HEC-2 (Reference 12) backwater analysis was started just upstream of the Brandon Lock and Dam in Joliet, Illinois. The slope-area method was used to establish the starting water-surface elevations.

For the areas studied by approximate methods, the elevation of the flood plain was determined from historic records and topographic maps developed by the Northeastern Illinois Planning Commission (Reference 8).

Flood profiles were drawn showing the computed water-surface elevations for floods of the selected recurrence intervals.

The hydraulic analyses for this study are based only on the effects of unobstructed flow. The flood elevations as shown on the profiles are, therefore, considered valid only if hydraulic structures, in general, remain unobstructed and if channel and overbank conditions remain essentially the same as ascertained during this study.

All elevations are referenced from National Geodetic Vertical Datum of 1929 (NGVD); elevation reference marks used in the study are shown on the maps.

4.0 FLOOD PLAIN MANAGEMENT APPLICATIONS

The National Flood Insurance Program encourages state and local governments to adopt sound flood plain management programs. Therefore, each Flood Insurance Study includes a flood boundary map designed to assist communities in developing sound flood plain management measures.

4.1 Flood Boundaries

In order to provide a national standard without regional discrimination, the 100-year flood has been adopted by FEMA as the base flood for purposes of flood plain management measures. The 500-year flood is employed to indicate additional areas of flood risk in the community. For each stream studied in detail, the boundaries of the 100-year and the 500-year floods have been delineated using the elevations determined at each cross section; between cross sections the boundaries were interpolated using topographic maps with an original scale of 1:24000 and a contour interval of 10 feet after being enlarged to a scale of 1:6000 (Reference 11).

The boundaries for the approximate studied streams were delineated on the above referenced maps.

The boundaries of the 100- and 500-year floods are shown on the Flood Boundary and Floodway Map. Small areas within the flood boundaries may lie above the flood elevations and, therefore, not be subject to flooding; owing to lack of detailed topographical information or to limitations of the map scale, such areas are not shown. In cases where the 100-year and the 500-year flood boundaries are close together, only the 100-year boundary has been shown.

4.2 Floodways

Encroachment on flood plains, such as artificial fill, reduces the flood-carrying capacity, increases the flood heights of streams, and increases flood hazards in areas beyond the encroachment itself. One aspect of flood plain management involves balancing the economic gain from flood plain development against the resulting increase in flood hazard. For purposes of the National Flood Insurance Program, the concept of a floodway is used as a tool to assist local communities in this aspect of flood plain management. Under this concept, the area of the 100-year flood is divided into a floodway and a floodway fringe. The floodway is the channel of a stream plus any adjacent flood plain areas that must be kept free of encroachment in order that the 100-year flood may be carried without substantial increases in flood heights. Minimum standards of FEMA limit such increases in flood heights to 1.0 foot, provided that hazardous velocities are not produced. However, under Chapter 19, Section 65F, Illinois Revised Statutes of 1973, encroachment in the flood plain is limited to that which will cause only an insignificant increase in flood heights (Reference 13). Thus, at the recommendation of the Illinois Division of Water Resources, a floodway having no more than a 0.1-foot surcharge has been delineated for this study.

The floodway for this study was computed on the basis of equal conveyance reduction from each side of the flood plain. The results of these computations were tabulated at selected cross sections for each stream segment for which a floodway was computed (Table 2). Portions of the Des Plaines River floodway lie outside the corporate limits.

As shown on the Flood Boundary and Floodway Map, the floodway widths were determined at cross sections; between cross sections, the boundaries were interpolated. In cases where the boundaries of the floodway and the 100-year flood are either close together or collinear, only the floodway boundary has been shown.

The area between the floodway and the boundary of the 100-year flood is termed the floodway fringe. The floodway fringe thus encompasses the portion of the flood plain that could be completely obstructed without increasing the water-surface elevation of the 100-year flood more than 0.1 foot at any point. Typical relationships between the floodway and the floodway fringe and their significance to flood plain development are shown in Figure 2.

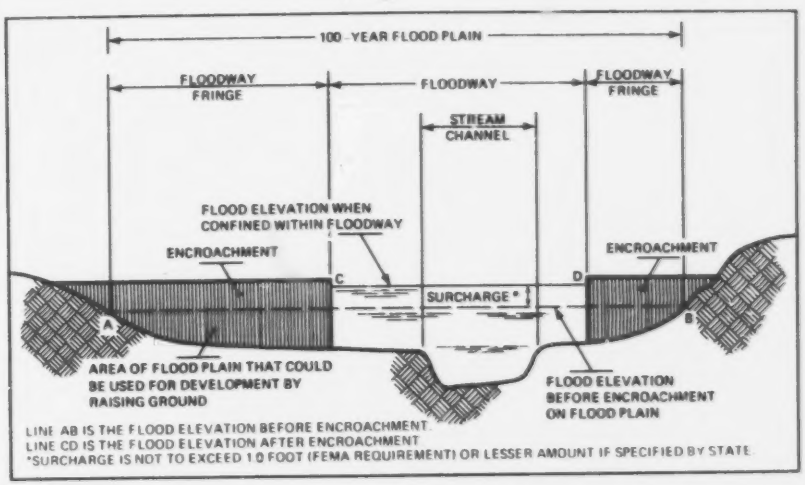
100-YEAR FLOOD PLAIN
FLOODWAY FRINGE
FLOODWAY
FLOODWAY FRINGE
STREAM CHANNEL
FLOOD ELEVATION WHEN CONFINED WITHIN FLOODWAY
ENCROACHMENT
ENCROACHMENT
C
D
SURCHARGE *
A
B
AREA OF FLOOD PLAIN THAT COULD BE USED FOR DEVELOPMENT BY RAISING GROUND
FLOOD ELEVATION BEFORE ENCROACHMENT ON FLOOD PLAIN
LINE AB IS THE FLOOD ELEVATION BEFORE ENCROACHMENT
LINE CD IS THE FLOOD ELEVATION AFTER ENCROACHMENT
*SURCHARGE IS NOT TO EXCEED 1.0 FOOT (FEMA REQUIREMENT) OR LESSER AMOUNT IF SPECIFIED BY STATE

FIGURE 2 - Floodway Schematic

The floodways in this report are recommended to local agencies as minimum standards that can be adopted or that can be used as a basis for additional studies.

5.0 INSURANCE APPLICATION

In order to establish actuarial insurance rates, FEMA has developed a process to transform the data from the engineering study into flood insurance criteria. This process includes the determination of reaches, Flood Hazard Factors (FHF), and flood insurance zone designations for each flooding source affecting the Village of Romeoville.

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FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION		
CROSS SECTION	DISTANCE[1]	WIDTH [2] (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC.)	WITH FLOODWAY (NGVD)	WITHOUT FLOODWAY (NGVD)	DIFFERENCE
DES PLAINES RIVER							
A	129,820	1752/1612	4446	2.07	585.8	585.7	0.1
B	130,990	2144/884	6687	1.38	587.5	587.0	0.1
C	132,370	1350/810	6367	1.44	587.8	587.4	0.1
D	133,670	1981/921	7275	1.26	587.8	587.7	0.1
E	135,320	1927/247	9388	0.98	588.1	588.1	0.0

[1]FEET ABOVE DRESDEN LOCK AND DAM
[2]TOTAL WIDTH / WIDTH WITHIN CORPORATE LIMITS

TABLE 2

FEDERAL EMERGENCY MANAGEMENT AGENCY

VILLAGE OF ROMEOVILLE, IL
(WILL CO.)

FLOODWAY DATA

DES PLAINES RIVER

5.1 Reach Determinations

Reaches are defined as lengths of watercourses having relatively the same flood hazard, based on the average weighted difference in water-surface elevations between the 10- and 100-year floods. This difference does not have a variation greater than that indicated in the following table for more than 20 per cent of the reach.

Average Difference Between 10- and 100-year Floods	Variation
Less than 2 feet	0.5 foot
2 to 7 feet	1.0 foot
7.1 to 12 feet	2.0 feet
More than 12 feet	3.0 feet

The locations of the reaches determined for the Village of Romeoville are shown on the Flood Profiles and are summarized in Table 3, "Flood Insurance Zone Data."

5.2 Flood Hazard Factors

The FHF is used to correlate flood information with insurance rate tables. Correlations between property damage from floods and their FHFs are used to set actuarial insurance premium rate tables based on FHFs from 005 to 200.

The FHF for a reach is the average weighted difference between the 10- and 100-year flood water-surface elevations expressed to the nearest 0.5 foot, and shown as a three-digit code. For example, if the difference between water-surface elevations of the 10- and 100-year floods is 0.7 foot, the FHF is 005; if the difference is 1.4 feet, the FHF is 015; if the difference is 5.0 feet, the FHF is 050. When the difference between the 10- and 100-year flood water-surface elevations is greater than 10.0 feet, accuracy for the FHF is to the nearest foot.

5.3 Flood Insurance Zones

After the determination of reaches and their respective FHFs, the entire incorporated area of the Village of Romeoville was divided into zones, each having a specific flood potential or hazard. Each zone was assigned one of the following flood insurance zone designations:

Zone A: Special Flood Hazard Areas inundated by the 100-year flood, determined by approximate methods; no base flood elevations are shown or FHFs determined.

Zone A2: Special Flood Hazard Areas inundated by the 100-year flood, determined by detailed methods; base flood elevations are shown, and zones subdivided according to FHF.

Zone B:	Areas between the Special Flood Hazard Area and the limits of the 500-year flood, including areas of the 500-year flood plain that are protected from the 100-year flood by dike, levee, or other water control structure; or areas subject to certain types of 100-year shallow flooding where depths are less than 1.0 foot; and areas subject to 100-year flooding from sources with drainage areas less than 1 square mile. Zone B is not subdivided.
Zone C:	Areas of minimal flooding.

Table 3, "Flood Insurance Zone Data," summarizes the flood elevation differences, FHFs, flood insurance zones, and base flood elevations for each flooding source studied in detail in the community.

5.4 Flood Insurance Rate Map Description

The Flood Insurance Rate Map for the Village of Romeoville is, for insurance purposes, the principal result of the Flood Insurance Study. This map (published separately) contains the official delineation of flood insurance zones and base flood elevation lines. Base flood elevation lines show the locations of the expected whole-foot water-surface elevations of the base (100-year) flood. This map is developed in accordance with the latest flood insurance map preparation guidelines published by FEMA.

6.0 OTHER STUDIES

A Flood Insurance Study is currently being conducted for the unincorporated areas of Will County and the City of Lockport (References 3 and 14) which are in agreement with this study.

In 1951, the Illinois Division of Waterways published a "Survey Report for Flood Control, Illinois and Michigan Canal and Tributaries, Joliet to Calumet-Sag Channel" (Reference 15). This report was based on a study of the flooding on the Illinois Michigan Canal and its tributaries in the reach between its junction with the Des Plaines River in the City of Joliet and the Calumet-Sag Channel. The report proposed several improvements to the canal. Those that would directly affect Romeoville included clearing and cleaning the canal and the diversion of Long Run Creek into the Chicago Sanitary and Ship Channel. The above report is in general agreement with this study.

The Flood Insurance Study completed for the City of Joliet, Illinois (Reference 8) matches this study.

A Flood Hazard Boundary Map has been published for the community (Reference 16). The differences between the Flood Hazard Boundary Map and this study are justified due to the more detailed nature of this Flood Insurance Study.

This report either supersedes or is compatible with all previous studies published on streams studied in this report and should be considered authoritative for the purposes of the National Flood Insurance Program.

FLOODING SOURCE	PANEL[1]	ELEVATION DIFFERENCE[2] BETWEEN 1.0% (100-YEAR) FLOOD AND			FLOOD HAZARD FACTOR	ZONE	BASE FLOOD ELEVATION[3] (NGVD)
		10% (10-YEAR)	2% (50-YEAR)	0.2% (500-YEAR)			
DES PLAINES RIVER							
REACH 1	0005	–0.9	–0.4	0.3	010	A2	VARIES–SEE MAP

[1]FLOOD INSURANCE RATE MAP PANEL
[2]WEIGHTED AVERAGE
[3]ROUNDED TO NEAREST FOOT

TABLE 3

FEDERAL EMERGENCY MANAGEMENT AGENCY

VILLAGE ROMEOVILLE, IL
(WILL CO.)

FLOOD INSURANCE ZONE DATA

DES PLAINES RIVER

BEST DOCUMENT AVAILABLE

7.0 LOCATION OF DATA

Information concerning the pertinent data used in preparation of this study can be obtained by contacting the Natural and Technological Hazards Division, Federal Emergency Management Agency, 300 South Wacker Drive, (24th Floor), Chicago, Illinois 60606.

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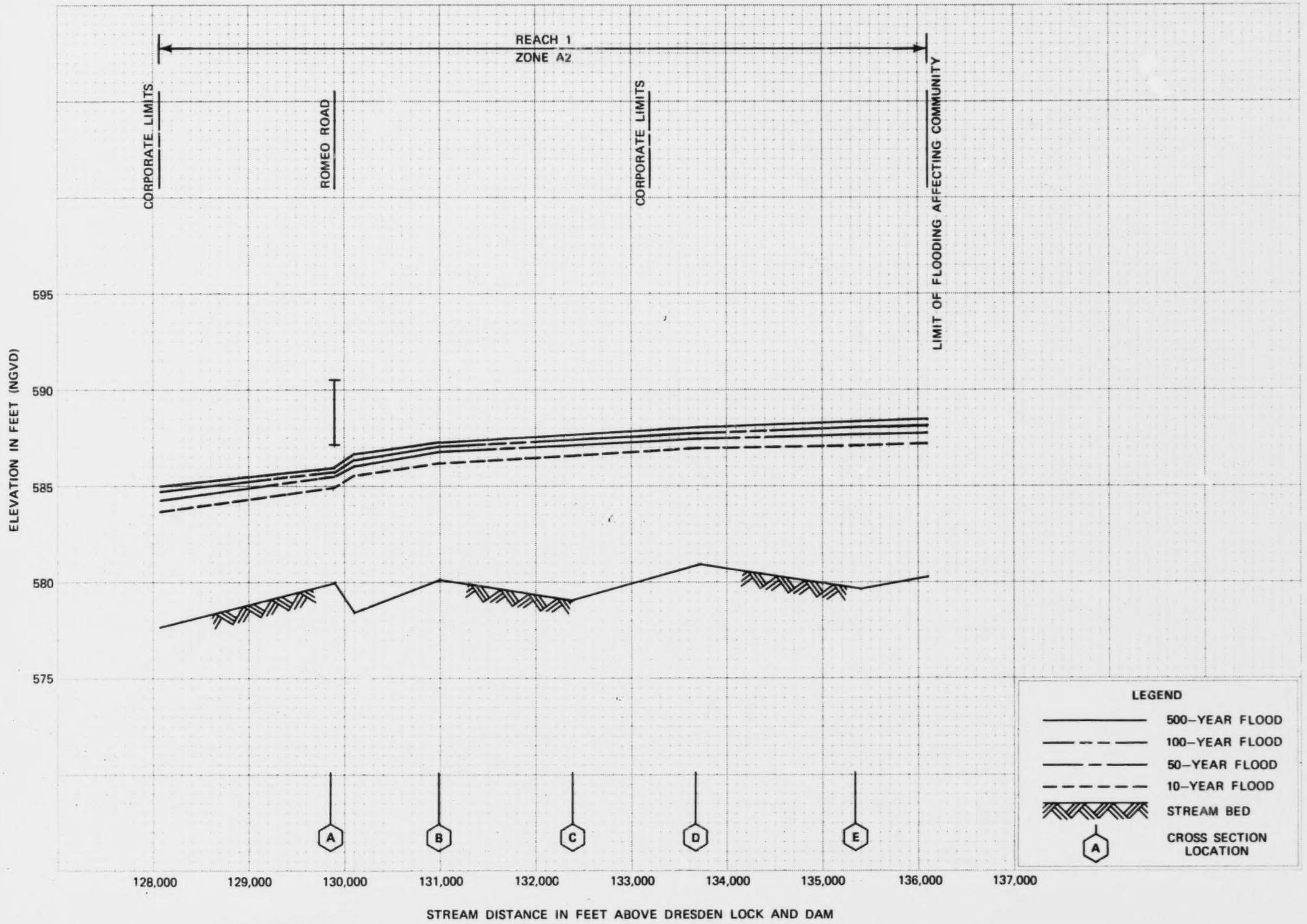

FLOOD PROFILES

DES PLAINES RIVER

FEDERAL EMERGENCY MANAGEMENT AGENCY

VILLAGE OF ROMEOVILLE, IL
(WILL CO.)

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